

GROUNDWATER EXPLORATION
CONSTRUCTION and TESTING REPORT

HORIZON HILLS BACK-UP WELL

Prepared for
Horizon Hills General Improvement District

TH #1-3	5948	
TH #4-5	4426	
Conductor	<u>13825</u>	24,199

Test well	12101	
Pumping	<u>6033</u>	18,134
		<u>42,333</u>

DRAFT

Prepared by
Washoe County Utility Division
January 1986

Contents

	Page
I Summary and Recommendations	1
II Introduction	2
III Exploratory Drilling	3
IV Test Well Construction and Development	6
V Pumping Tests	8
A. Step Drawdown Test	8
B. Constant Discharge Test	13
C. Recovery Test	14
D. Results and Discussion	17
VI Water Quality	21
Appendix I Water Well Drillers Report	
II Pumping Test Data	
III Water Quality Analysis	

List of Tables and Figures

Tables

	<u>Page</u>
1 Step Drawdown Tests - Specific Capacity	10
2 Calculated Drawdown and Well Efficiency	13
3 Time Series for Iron and Manganese	22
4 Water Quality Analysis for New and Present Well with SDWA Primary Standards	23

Figures

	<u>Page</u>
1 Lithologic Log for Exploration Hole No. 3	5
2 As-Built Construction Details	7
3 Step Drawdown Test	9
4 Specific Discharge vs. Well Yield Data from Step Drawdown Test	11
5 Formation and Well Loss - Efficiency Diagrams	12
6 Constant Discharge - Drawdown Test	15
7 Recovery Data	16
8. Constant Discharge vs Recovery	19

Summary and Recommendations

1. Exploratory test holes were drilled up to 400 feet in fractured rhyolite and/or metavolcanics. Extensive fracturing occurs in the interval 280-380 feet below land surface. This interval appeared to have adequate water producing potential.
2. An 8-inch test well with 165 feet of 16-inch conductor casing was installed to 396 feet. A sanitary seal around the conductor casing was installed to a depth of 165 feet. The well has triple mill-slot casing from 270 feet to 390 feet. This well produces from fractures in the bedrock.
3. A step drawdown test determined the well to be inefficient at moderate pumping rates. The inefficiency is due to large friction losses in the fractures that serve as an extended well rather than the well construction itself. A constant discharge test was run at 209 gpm and 186 gpm for 72 hours. Drawdown rates never stabilized and at least two impermeable boundaries or dewatered fractures were encountered.
4. The water quality was generally good except for iron and manganese. During the pumping test manganese levels remained constant at 0.22 ppm while iron levels dropped from 0.94 ppm to 0.42 ppm. Secondary drinking water standards for iron and manganese are 0.60 and 0.1 respectively.
5. This well could be equipped with a 150 gpm pump and used as an alternate supply to the existing well. Pumping should never exceed an average of 80 gpm on a continuous basis. Water treatment is necessary to meet State requirements for iron and manganese.

INTRODUCTION

The Horizon Hills General Improvement District Board of Trustees authorized the Washoe County Utility Division to conduct an investigation in locating a secondary source of water supply for its subdivision. A "back-up" well is necessary to comply with State regulations and County codes for subdivision water supplies. The goal of the investigation was to locate a water source that would meet the water quantity and quality requirements of the subdivision. This report documents the investigation, which included exploratory drilling, test well construction and aquifer testing.

The location of Horizon Hills Subdivision at the base of Peavine Mountain lends itself to groundwater quality and quantity problems. The highly mineralized and hydrothermal altered rock of Peavine Mountain causes mineralized water, specifically dissolved iron and manganese. Because there are no sizeable alluvial aquifers or surface streams in the vicinity, fractured bedrock is the only reliable source of water supply. However, locating such a well is quite often a risk. Washoe County chose a site for exploratory drilling in an area close to the subdivision in a rhyolite plug that appeared highly fractured and without evidence of hydrothermal alteration.

A contract was awarded to Sargent Irrigation in January 1985 for an exploratory test hole and air lift pumping test to determine water quality and quantity. Because of the highly fractured nature of the geology, and favorable indications for water quality and quantity, an 8-inch test well was constructed and tested. A pumping test was completed in December 1985 and the results, analysis and recommendations are included herein.

Exploratory Drilling

The exploratory drilling contract called for one 500 foot test hole. Five test holes were eventually drilled. The following is a synopsis of each test hole.

Test Hole No. 1 was drilled to 200 feet with a reverse rotary air drill rig. Alternating lenses of iron stained rhyolite and clay were encountered. The project geologist (Michael Widmer) terminated the test hole at 200 feet because of unfavorable hydrogeologic conditions.

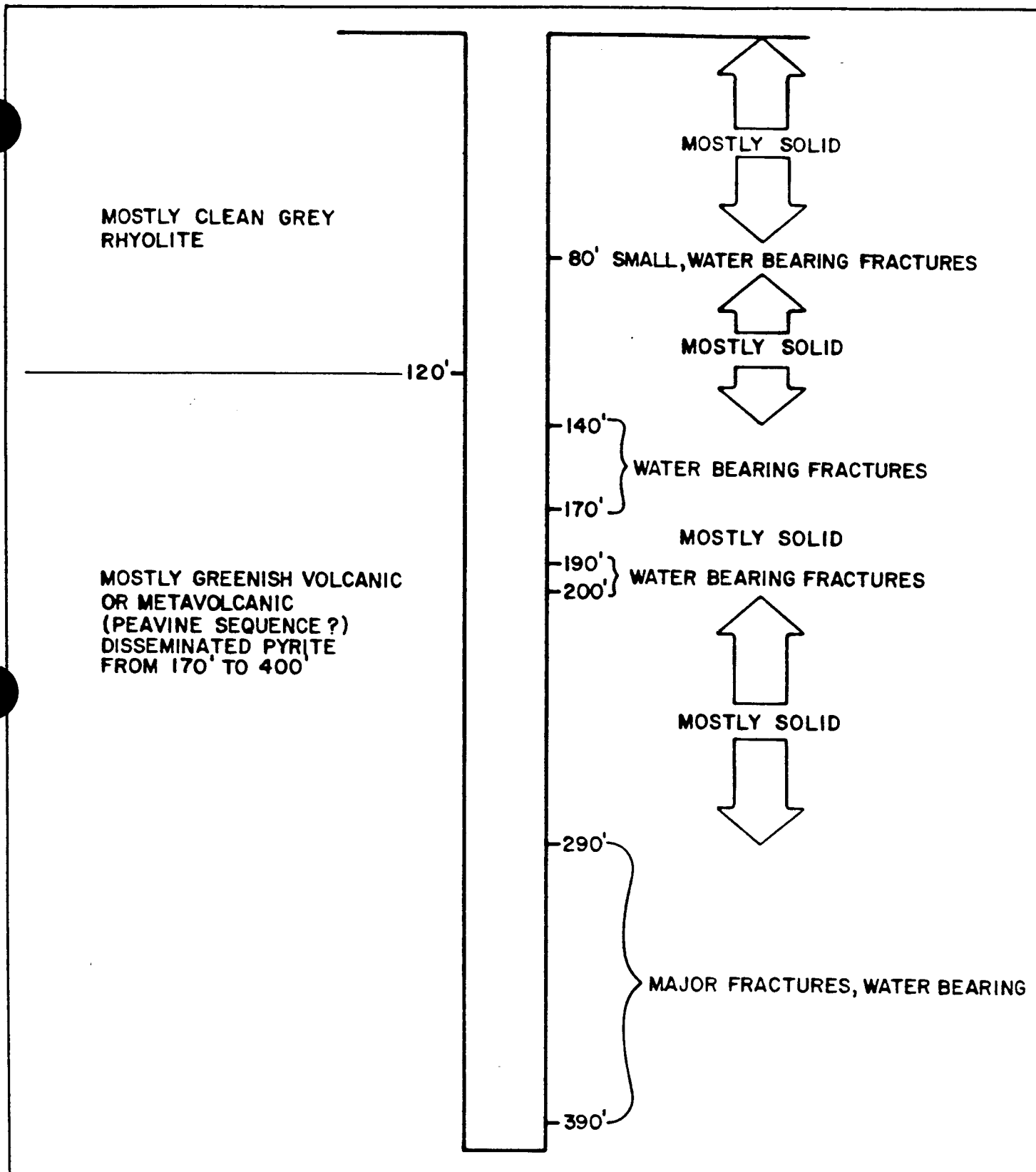
Test Hole No. 2 was drilled approximately 200 feet to the south of Test Hole No. 1 and drilled to a depth of 350 feet before the rollers of the tri-cone drill bit were lost in the hole. This test hole encountered highly fractured rhyolite and metamorphosed volcanics with apparent good water bearing potential.

Test Hole No. 3 was drilled approximately 6 feet to the east of Test Hole No. 2 to a depth of 403 feet. The same geology and water bearing fractures as in test hole No. 2 were encountered. This test hole was drilled air-reverse with a hammer bit and tri-cone bits. Several bits were ruined due to the hardness and fractured state of the geology. The hammer bit became inoperable below 300 feet due to excessive water pressure. A mud rotary rig was set up over the hole and never drilled deeper than 80 feet because of lost circulation problems. This circulation problem was such that the drilling mud flowed into non-water bearing fractures at a greater rate than drilling mud could be provided at the surface. Cement was used to try and plug the fractures, but was unsuccessful.

Test Hole No. 4 was drilled to 80 feet by a mud rotary rig and encountered the same problems of lost circulation. This test hole was located 25-feet from Test Hole No. 3.

Test Hole No. 5 was drilled to 300 feet by the air-reverse rotary rig with an air hammer bit. Excessive water pressures inhibited the hammer bit operation below 300 feet. Sargent decided not to drill below 300 feet with a tri-cone bit.

Based on the information obtained from the exploratory drilling, it was agreed upon to drill and construct a test well to 400 feet with a reverse rotary mud rig. The lost circulation zones would be circumvented by installing a 16 inch conductor casing to 165-feet with a cable tool drill rig. A lithologic log is shown in figure 1. Test holes 2-5 all encountered the same geology. Test holes 1, 2, 4, and 5 were plugged with neat cement.



WASHOE COUNTY DEPARTMENT OF PUBLIC WORKS

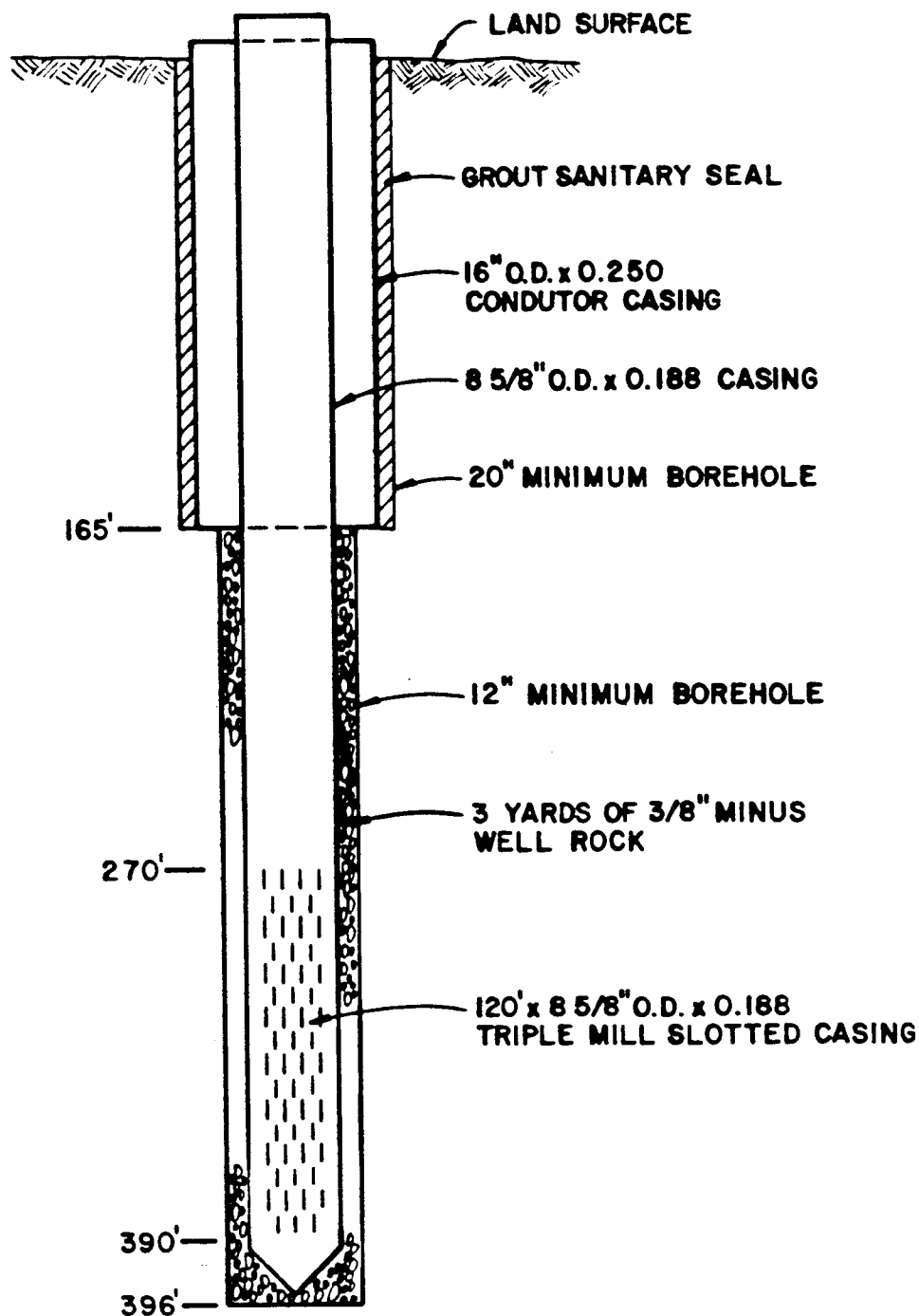
UTILITY DIVISION		FIGURE 1		SHEET NO 1 of 1 SHEET(S)
DATE	JAN. 1986	BY	MCW / JR	
Scale	NONE	APPROVED		
REVISIONS	DATE	LITHOLOGIC LOG EXPLORATION HOLE # 3		

Test Well Construction and Development

Construction of a test well began August 8, 1985 in Test Hole No. 3 with the drilling of a 20 inch hole by a cable tool rig. A 16-inch conductor casing was inserted to a depth of 165 feet and completed September 11th. On September 12th, eight yards of grout (neat cement) was used for a sanitary seal to a depth of 165 feet to land surface. This conductor casing work was subcontracted to MacKay Drilling.

On October 23rd, Sargent Irrigation set-up a reverse rotary mud rig and began drilling a 12 inch borehole in Test Hole No. 3. The borehole was completed to a depth of 400 feet on October 31st. Well construction was completed November 1st (see figure 2). Two fifty pound bags of sodium acid-pyrophosphate were added to the well to help breakdown the drilling fluids (bentonite clay mud). Four hours of well development (pumping and air-lifting) occurred on November 1st and 2nd. On November 4th, three yards of pea gravel was installed in the well-borehole annular space to serve as a formation stabilizer. Downhole problems prevented a drawdown tube from being included in the well construction.

Four hours of additional well development occurred on December 15th. A turbine line-shaft pump was installed to a depth of 350 feet. The well was pumped at rates varying from 160 gpm to 240 gpm with periods of surging. This pumping and surging (turning pump off to create a backwash effect) helps to remove sediment from the well and to create a better filter from the gravel pack or formation stabilizer. Drilling fluids are also removed from the well/aquifer. After development the water was clear and had very little sediment.



WASHOE COUNTY DEPARTMENT OF PUBLIC WORKS

REVISIONS	DATE	UTILITY DIVISION		FIGURE 2	SHEET NO 1 OF 1 SHEET(S)
		DATE JAN. 1986	BY MCW / JR	AS BUILT CONSTRUCTION DETAILS	
		Scale NONE	APPROVED		

Pumping Test

Two pumping tests were conducted from December 16th to December 20th. The line-shaft turbine test pumping equipment consisted of 350 feet of 4-inch pump column with twelve stages of 7 inch bowl assembly and engine. The discharge rate was measured by a 4.124-inch I.D. horizontal discharge pipe with a 3.375-inch I.D. orifice plate. A manometer was attached to the discharge pipe and adjustments to flow were made by a gate valve or engine r.p.m. adjustments. Water level measurements were made via a drawdown tube provided to a depth of 333 feet and were made to the nearest 1/100 foot.

Step-Drawdown Test

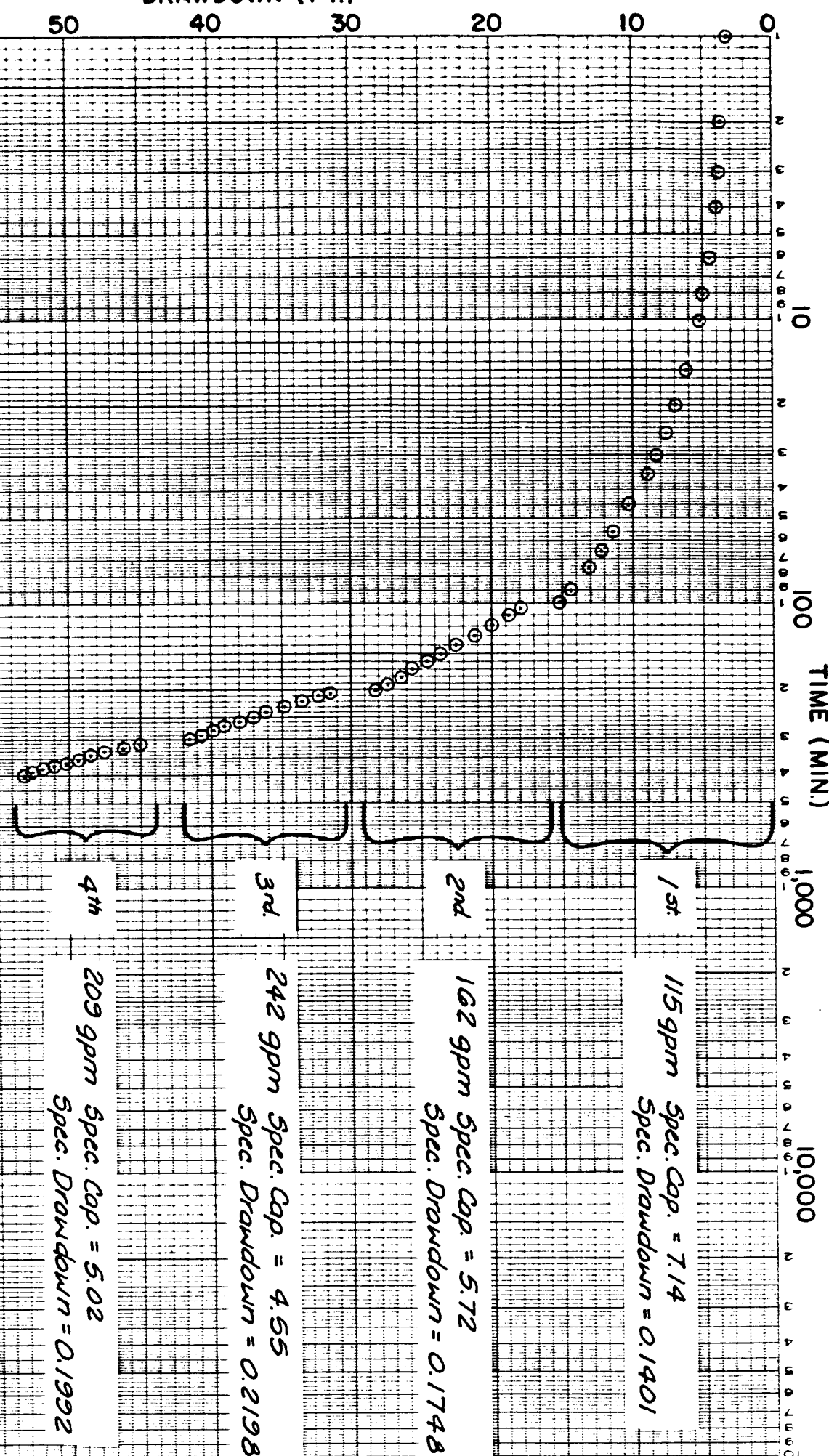
A step-drawdown test is used to determine well efficiency, well yield, specific capacity and pumping levels at various pumping rates. Various and increasing pumping rates are established for a minimum period of 100 minutes with a minimum of three periods. This test was conducted December 16th for 400 minutes (6 2/3 hours) at pumping rates of 115 gpm, 162 gpm, 209 gpm and 242 gpm. The data is shown in figures 3-5.

Figure 3 shows the drawdown in the well versus time and pumping rate. For each step the pumping rate is noted as well as the specific capacity for each step. Specific capacity describes the well capability to pump water at a pumping rate per foot of drawdown in the well. The drawdown in a well is a function of head losses in the well and head losses in the aquifer formation and the pumping rate - this in turn is used to determine its efficiency. It is standard practice to determine these losses by the equation:

$$S_w = BQ + CQ^2 \quad (1)$$

where S_w is the drawdown in the well, Q is the pumping rate or discharge, B is the coefficient of well loss and C is the coefficient of formation loss. When the specific drawdown (inverse of specific capacity) is plotted versus

FIGURE 3
HORIZON HILLS WELL #2
STEP-DRAWDOWN TEST
DEC. 16, 1985



discharge, a straight line can be drawn through these points as depicted in figure 4. The slope of this line is equal to the formation loss coefficient and the line intercept with the graph is equal to the well loss coefficient. Table 1 shows the calculations used to determine these coefficients.

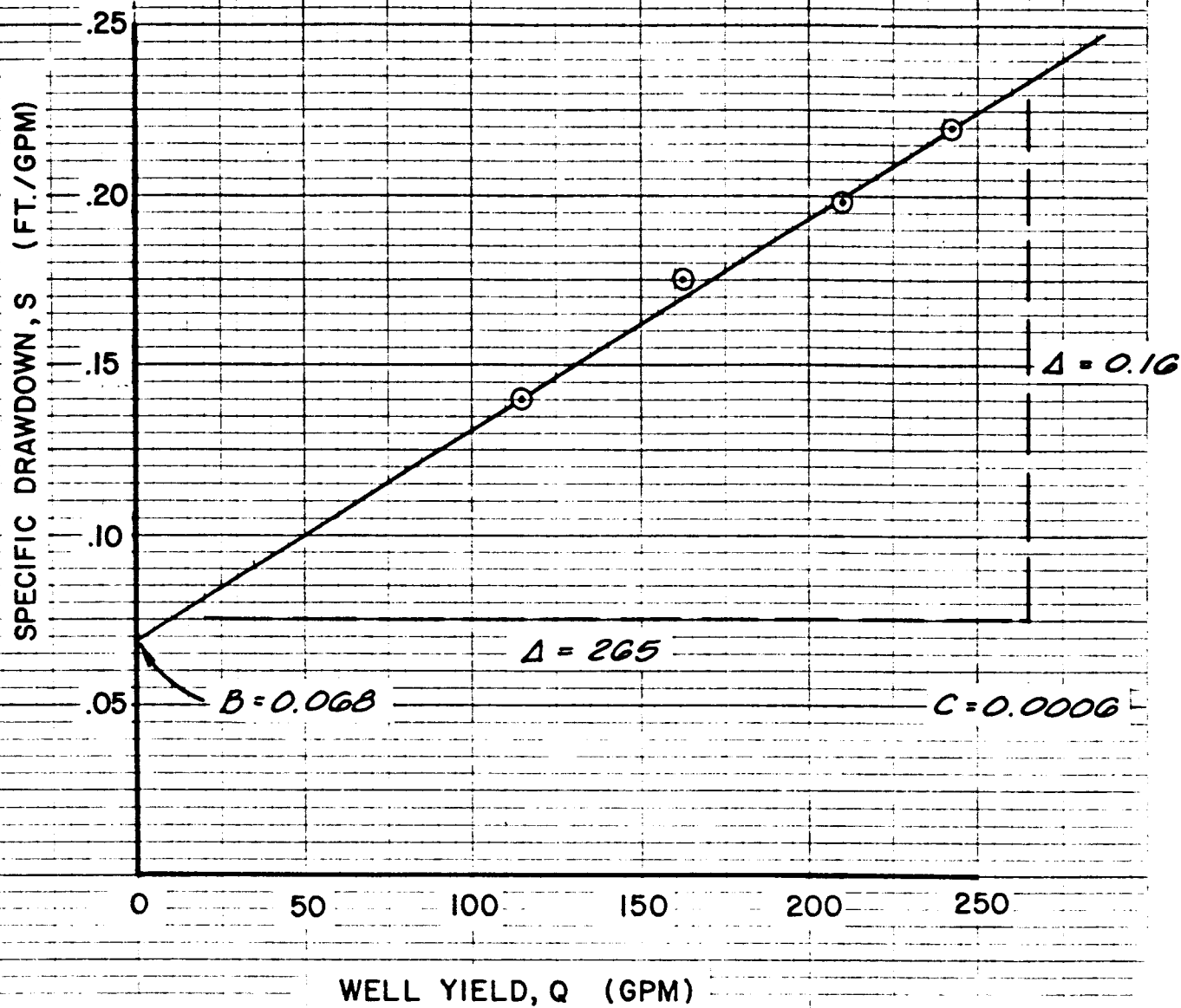
Table 1				
Step Drawdown Tests - Specific Capacity				
Step	Q Well Yield (gpm)	S _w Drawdown after 100 min. (ft)	Q/S Specific Capacity (gpm/ft)	S/Q Specific Drawdown (ft/gpm)
1	115	15.27	7.14	0.1401
2	162	28.30	5.72	0.1748
3	209	41.65	5.02	0.1992
4	242	53.16	4.55	0.2198

Well efficiency is ultimately described by this graphical method from the equation

$$E = \frac{1}{1 + (C/B) Q} \quad (2)$$

Table 2 shows these calculations and figure 5 graphically displays the results.

FIGURE 4
SPECIFIC DISCHARGE VERSUS WELL YIELD
DATA FROM STEP-DRAWDOWN TEST



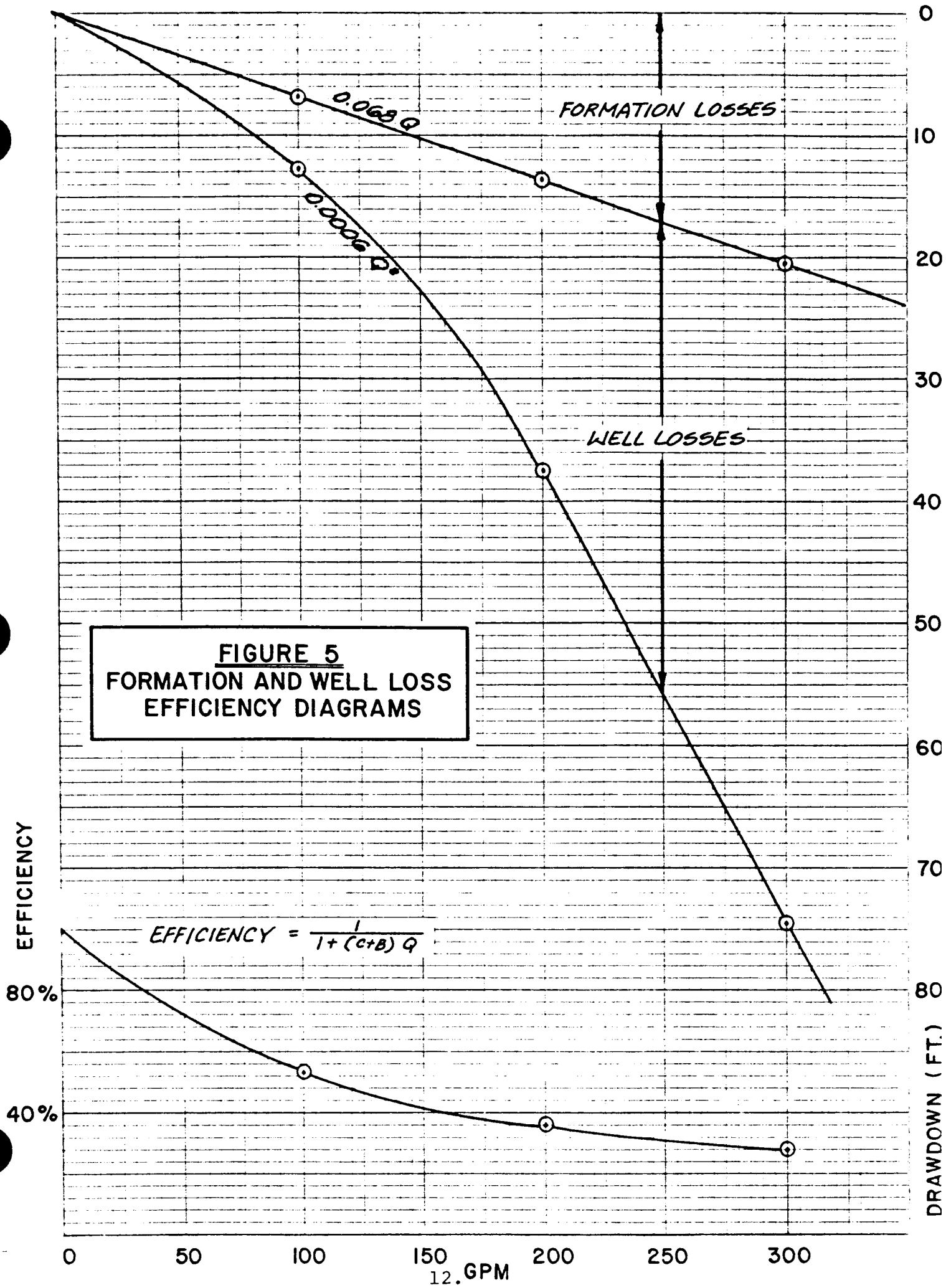


Table 2		
Calculated Drawdown and Well Efficiency		
Q Well Yield (gpm)	S _w Calculated Drawdown (ft)	E Well Efficiency (%)
100	12.8	53
200	37.6	36
300	74.4	27
$S_w = (0.068)Q + (0.0006)Q^2$ $E = \frac{1}{1 + (C/B) Q}$		

As can be seen in figure 5, well losses account for most of the drawdown as the discharge rate is increased. Well losses should be a minor portion of the total drawdown. These losses reflect the construction of the well among other things. At the designed pumping rate of the well, efficiencies should be in the range of 70-90%. As can be seen from the graph, an efficiency of only 42% is achieved at a pumping rate of 150 gpm. The physical reason for such low efficiencies and high well losses stems from the well penetrating fractured bedrock. These water bearing fractures actually serve as an extension of the well itself. Such low efficiencies typify fracture flow wells. The efficiency of this well may initially increase due to more development of the fractures through pumping. Well efficiency should be tested on an annual basis in order to insure proper maintenance and longevity of the well.

Constant Discharge Test

A constant discharge test is run in order to determine aquifer parameters and discover any boundary conditions. Aquifer parameters describe how well the aquifer transmits water and its storage capabilities. Transmissibility

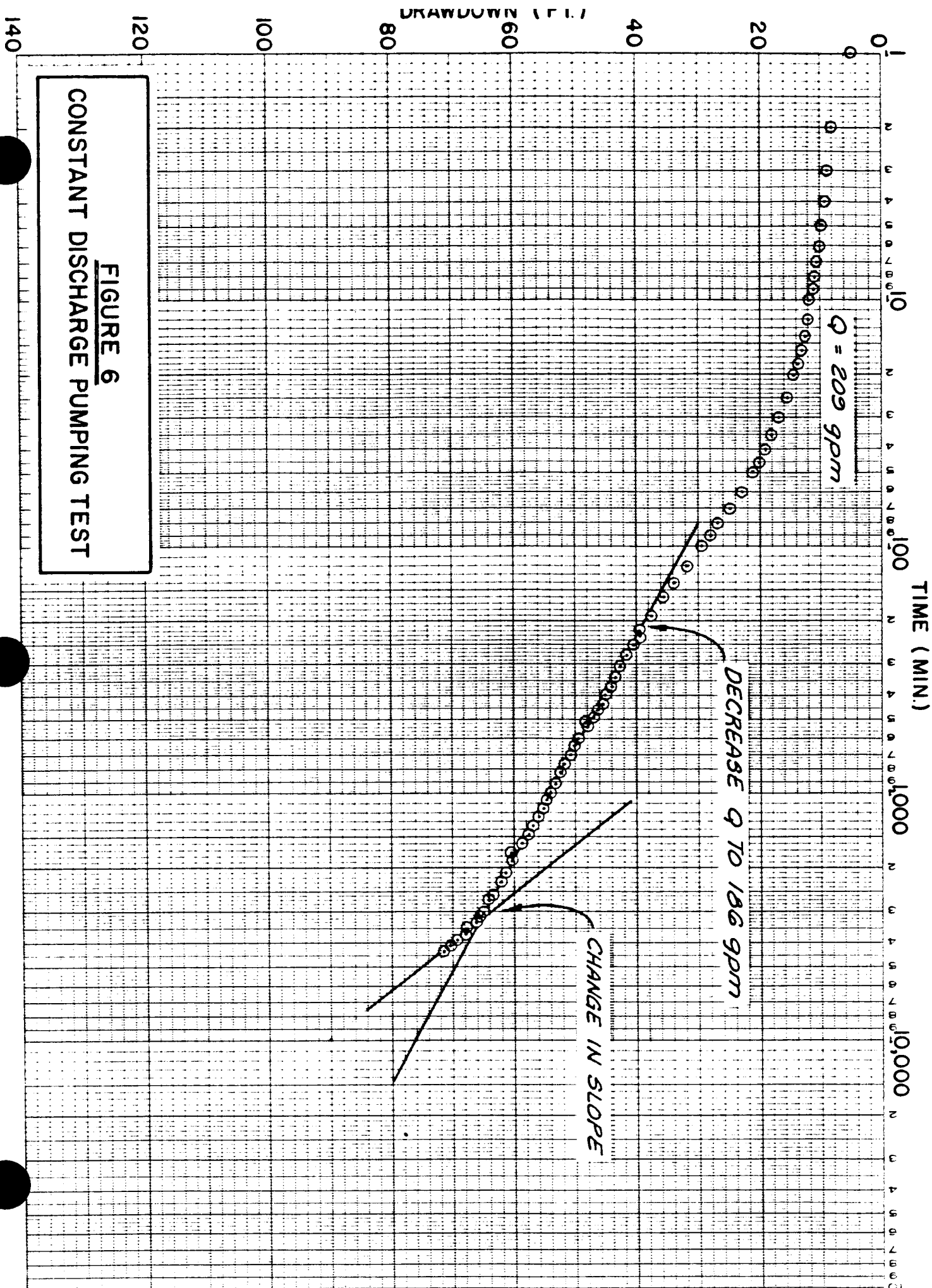
is used to predict future drawdowns or pumping levels in the well. Obviously, for a well being used for twenty years or longer, this information is invaluable. Boundary conditions such as faults (impermeable boundary) or large bodies of water - rivers, lakes and streams (recharge boundary) have a cause and effect on the aquifer's natural condition and flow pattern. A pumping well's drawdown rate will be effected by these boundary conditions with either an increase or decrease in drawdown over time. An increase in the drawdown rate infers that an impermeable boundary of the aquifer has been reached by the well drawing water from the aquifer. In a fracture flow well, this may mean that certain fractures may be entirely dewatered.

On December 17th, a 72-hour pumping test was initiated. A pumping rate of 209 gpm was chosen, but later reduced to 185 gpm because the pump was not capable of sustaining the higher pumping rate. Figure 6 shows the drawdown in the well versus time.

The early time data (20 minutes) shows a low drawdown rate before an increased, but constant drawdown rate occurs. This probably results from dewatering small upper fractures and storage effects. An increase in drawdown then results from the constant discharge drawing more water from the main production zone of fractures (280 feet - 380 feet). At 3500 minutes (2 1/2 days), an increase in the drawdown rate occurs. This doubling of the drawdown rate reflects an impermeable boundary where no flow occurs. Because of the nature of this fracture flow aquifer, it probably reflects the actual dewatering of a larger fracture.

Recovery Data

The recovery test occurs after the constant Q pumping test is ended and the pump is turned off. Water level measurements are taken at specific time



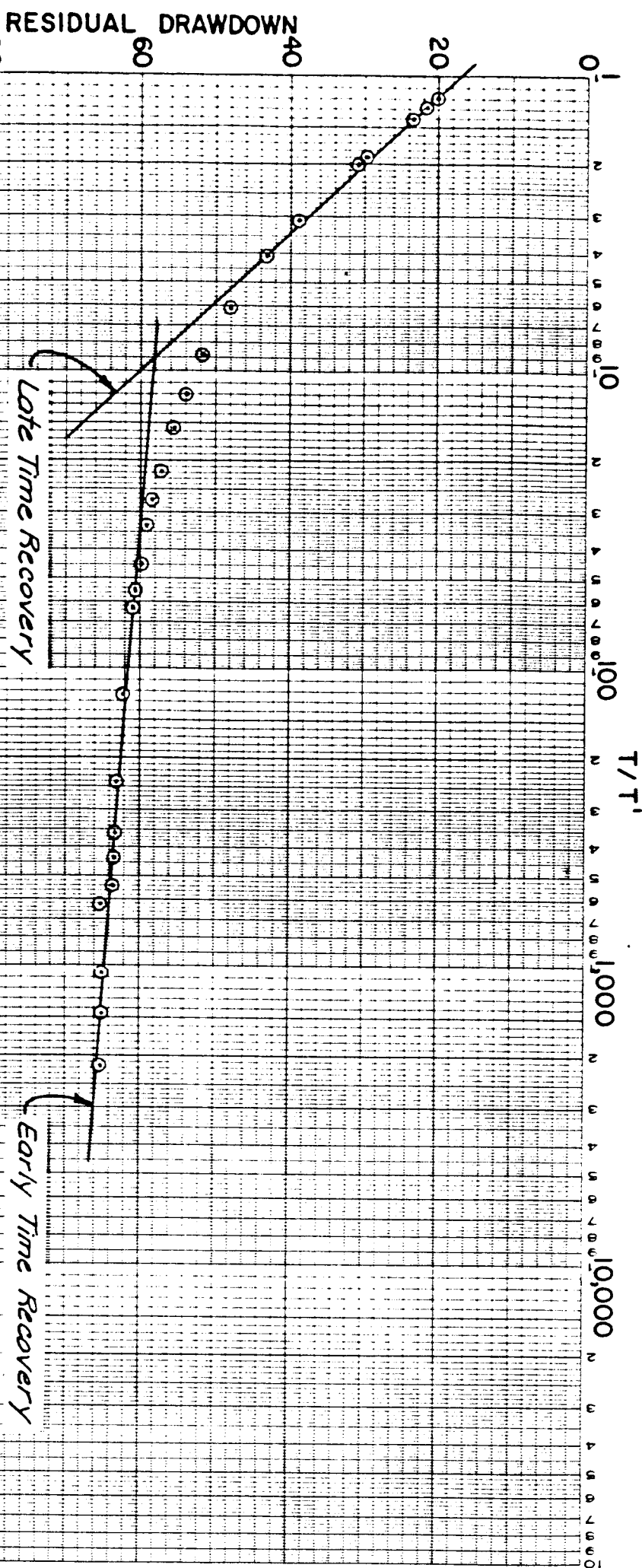


FIGURE 7
RECOVERY TEST

intervals to determine the rate at which the aquifer recovers. A plot of these data should be a mirror image of the pumping test. Figure 7 shows these results. Theoretically, the static water level should return to its original level after 72 hours. Figure 7 shows that at 72 hours after pumping stopped ($t/t' = 2$), there was still 30 feet of residual drawdown in the well. This indicates fractures were dewatered and could not contribute flow to the well as initially. After two weeks of recovery, the well still has 20 feet of residual drawdown.

The results of this test show that the well derives its water from fractures and that some fractures have been dewatered. A considerable amount of time will be required for the well to fully recover.

Results and Discussion

The step drawdown test has shown the well to be inefficient, especially at high pumping rates. The inefficiency results from high friction losses associated with fracture flow. The fractures can be considered extension of the well itself so that these friction losses show up as well losses. At a pumping rate of 150 gpm, the formation losses are nearly equal to the well losses. The efficiency of the well at this pumping rate is approximately 45%.

The constant discharge test clearly simulates fracture flow theory. During the 72 hours of pumping, the rate of drawdown remained constant or increased due to the dewatering of fractures. A total of 72 feet of drawdown occurred during this test which represents approximately 40% of available drawdown. At this rate the drawdown would reach a pumping level of 350 feet after approximately six years of constant pumping. This is also in the absence of any additional dewatering of fractures or boundary conditions and in absence of recharge.

Long term predictions of drawdown cannot be made based on the limited amount of data available. The results of the 72 hour test indicate the well would likely be subject to continually changing conditions and therefore be impossible to evaluate for long term yield. This differs from alluvial wells which are much more predictable because of the predictability of the aquifer media. For fracture flow wells, a conservative method of analysis is to plot drawdown vs the square root of time as shown in figure 8. According to this plot, the drawdown would reach a 350 foot pumping level in approximately 100 days. However, since this well would not be pumped continuously, periods of recovery would occur as figure 8 depicts. Based on intermittent pumping as shown in figure 8, drawdown to 350 feet is reached in approximately one year. However, the above prediction is conservative because it does not consider recharge which most certainly occurs and would reduce the rate of drawdown. Transmissibility and storativity could not be determined because observation wells were not available.

An additional consideration is that before the four hours of development occurred, the static water level in the well was 131.5 feet. Eleven hours after development the static had recovered to 143.2 feet. After the step drawdown pumping test (6 3/4 hours) and about eighteen hours of recovery, the static level was 151.5 feet. Two weeks after the constant discharge test, the well's static was 172 feet; 40 feet below the original static. This indicates that probably some upper level fractures had been dewatered and no longer contributes flow to the well. These fractures are probably located above 170 feet (see Lithologic Log). Consequently, the practical static water level is closer to 170 feet below land surface.

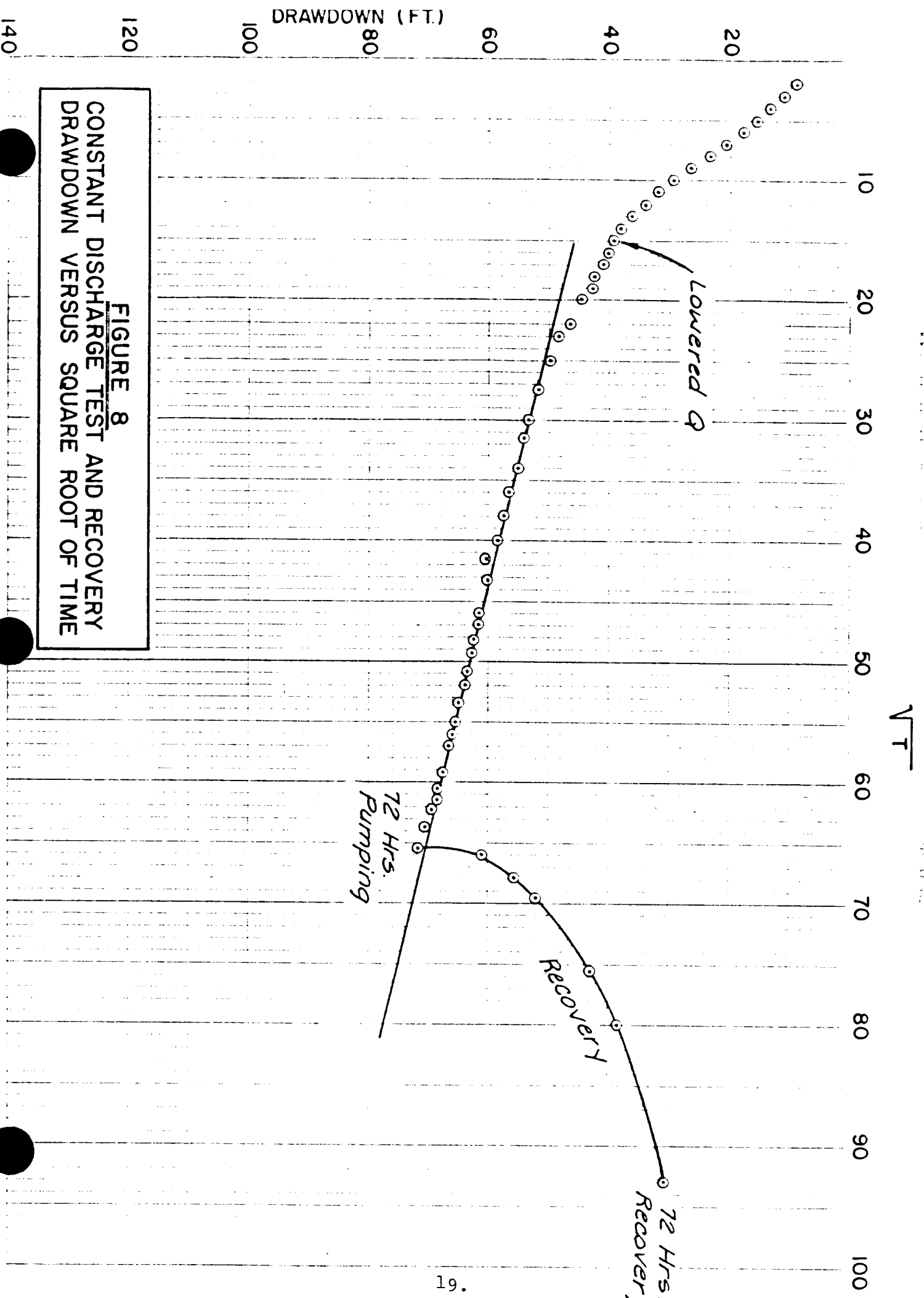


FIGURE 8
CONSTANT DISCHARGE TEST AND RECOVERY
DRAWDOWN VERSUS SQUARE ROOT OF TIME

What is important is matching the needs of the community to the well's capability. On a monthly basis, Horizon Hills demands between 15 gpm in the winter to 130 gpm in the summer. Peak demands have not been determined. The existing well is equipped to pump 225 gpm. Based on this limited information, a yearly average pumping rate of 55 gpm is used by the subdivision or approximately 0.45 gpm/unit. For comparison, Lemmon Valley Water Company currently supplies 0.65 gpm/unit. At this higher rate, Horizon Hills would need 82.5 gpm/year. This higher demand would probably occur if water quality improved.

Based on this information, it is reasonable to equip this well to pump a maximum of 150 gpm with an average annual usage of 80 gpm. Because of the uncertainty of its long term yield, the well should only be used for back up purposes until additional monitoring provides the data necessary to determine the well's long term yield.

Water Quality

A major concern with this well is water quality. A problem with iron, manganese and turbidity exists with the current well. It was hoped that these problems would be solved with this new water supply source. In much of the lithology, pyrite was disseminated throughout. Pyrite is composed of iron and sulfur.

During the constant discharge test and most of the step drawdown test, the discharge was clear and free of sediment. The temperature stayed constant at 58⁰ F. Electrical conductivities generally decreased from 500 micromhos to 470 micromhos. Water samples were taken at twelve hour intervals for iron and manganese analysis. These results are shown in Table 3 and indicate a general decrease in iron from 0.94 to 0.42 ppm. Manganese is fairly constant at 0.22 ppm. The primary drinking water standards for iron and manganese are 0.3 ppm and 0.05 ppm, respectfully. Secondary standards are 0.6 ppm and 0.1 ppm. These results show iron within secondary standards and manganese exceeding secondary standards.

During the pumping test, the Washoe County District Health Department sampled for all constituents after 56 and 72 hours of pumping. These results are shown in Table 4. Also listed, are results of analysis from the primary well taken in November 1985. The overall quality of the new well water is good with mostly ions of sulfate, bicarbonate, calcium, magnesium and sodium. Total dissolved solids are well within primary standards. This water is considered very hard with respect to calcium and manganese.

In comparison to the present well, the quality of water from the new well is slightly better in nearly all constituents. However, iron and manganese still remain a problem and exceed primary and/or secondary standards. Consequently, these waters do not meet with Washoe County District Health Department standards. If either of these waters are to be used for domestic purposes, they must be treated for iron and manganese.

Table 3			
Sample Identification		Parameter	Parameter
Sample Collection Date	Time (0-2400)	Iron Units mg/l	Manganese Units mg/l
12-17-85	0845	0.94	0.23
12-17-85	1940	0.48	0.20
12-18-85	0810	0.55	0.20
12-18-85	2100	0.54	0.22
12-19-85	0800	0.55	0.22
12-19-85	2130	0.50	0.22
12-20-85	0750	0.42	0.22

Water quality analysis for new and present well with SDWA Primary Standards expressed in p.p.m.

* Expressed in standard units.

Table 4			
Constituent	New Well	Present Well	SDWA Primary Standards
TDS	372	428	500
*Hardness	228	265	80-100
Calcium	50	60	---
Magnesium	25	28	125
Sodium	25	22	---
Potassium	1	2	---
Sulfate	179	190	250
Chloride	2	5	250
Nitrate	0.4	0.3	10-45
*Alkalinity	92	122	---
Bicarbonate	112	149	---
Fluoride	0.27	0.17	1.4-2.4
Arsenic	<0.003	0.001	0.05
Iron	0.42-0.94	0.26-1.2	0.3
Manganese	0.20-0.23	0.15-0.29	0.05
Copper	0.01	0.01	1.0
Zinc	0.02	0.01	5.0
Barium	0.02	0.01	1.0
Boron	0.0	0.0	---
Silica	30	34	---
*Color	7	7-45	15
*Turbidity	1.5	2.1	---
*pH	7.54	7.54	6.5-8.5

Log No.
Permit No.
Basin

WELL DRILLERS REPORT

PRINT OR TYPE ONLY

Please complete this form in its entirety

HORIZON HILLS GENERAL IMPROVEMENT DISTRICT
OWNER c/o WASHOE COUNTY DEPT PUBLIC WORKS ADDRESS AT WELL LOCATION
MAILING ADDRESS 1205 MILL STREET Southwest Horizon Hills 1 mile
RENO, NEVADA 89520

2. LOCATION SE ¼ SE ¼ Sec. 17 T. 20 N. S. R. 19 E. WASHOE County
 PERMIT NO. W-55/App. 48567
 Issued by Water Resources Parcel No. Subdivision Name

3.	TYPE OF WORK		4.	PROPOSED USE		#4	5. TYPE WELL	
	New Well <input checked="" type="checkbox"/>	Recondition <input type="checkbox"/>		Domestic <input type="checkbox"/>	Irrigation <input type="checkbox"/>		Test <input checked="" type="checkbox"/>	Cable <input type="checkbox"/> Rotary <input checked="" type="checkbox"/>
	Deepen <input type="checkbox"/>	Other <input type="checkbox"/>		Municipal <input type="checkbox"/>	Industrial <input type="checkbox"/>		Stock <input type="checkbox"/>	Other <input type="checkbox"/>

6. LITHOLOGIC LOG

[illegible]

Date started.....JUNE 19....., 1985
Date completed.....JUNE 24....., 1985

7. WELL TEST DATA

Pump RPM	G.P.M.	Draw Down	After Hours Pump

BAILER TEST

G.P.M. _____ Draw down _____ feet _____ hours
G.P.M. _____ Draw down _____ feet _____ hours
G.P.M. _____ Draw down _____ feet _____ hours

8. WELL CONSTRUCTION

Diameter hole 8 inches Total depth 80 feet

Casing record.....
Weight per foot..... Thickness.....

[illegible]

Surface seal: Yes ☒ No ☐ Type cement grout

Depth of seal abandoned feet

Gravel packed: Yes ☐ No ☐

Gravel packed from.....feet to.....feet

Hole caved-in to 50 feet - Water table filled
Perforations: with natural materi

Type perforation.....

Size perforation

From.....feet to.....feet

From.....feet to.....feet

From _____ feet to _____ feet

From _____ feet to _____ feet

From _____ feet to _____ feet

.....

9. WATER LEVEL

Static water level feet below land surface

Flow.....G.P.M.....P.S.I.

Water temperature° F. Quality

10. DRILLERS CERTIFICATION

This well was drilled under my supervision and the report is true to the best of my knowledge.

Name CHARLES SARGENT IRRIGATION, INC.
Contractor

Address P. O. BOX 2480 RENO, NEVADA 89505

Nevada contractor's license number 21246

Nevada contractor's drillers number.....1392

Nevada driver's license number 1413

Signed Manuel Veliz Actual Driller

Contractor _____
Date August 10, 1985



WASHOE COUNTY

DEPARTMENT OF PUBLIC WORKS
UTILITY DIVISION

PUMPING TEST DATA

WELL Horizon Hills

☒ PUMPING/OBSERVATION WELL
☐ PUMPING/RECOVERY DATA

PAGE 1 OF 2

TYPE OF PUMPING TEST Step Drawdown

HOW Q MEASURED orifice plate / manometer

HOW WL's MEASURED elec. sounder

M.P. for WL's _____ elev. _____

DEPTH of PUMP/AIRLINE 350 ft wri _____

PUMPED WELL NO. _____ % SUBMERGENCE: initial _____; pumping _____

RADIUS of PUMPED WELL _____ PUMP ON: date 12/16/85 time 0814

DISTANCE from PUMPED WELL _____ PUMP OFF: date 12/16/85 time 1455

TIME					WATER LEVEL DATA					WATER PRODUCT.		COMMENTS
at t'=0					STATIC WATER LEVEL							
CLOCK TIME	ELAPSED TIME		t/t'		READING	CONVERSIONS or CORRECTIONS	WATER LEVEL	S or S'		Q	(NOTE ANY CHANGES BY OBSERVERS)	
0814	/	0			143.22		139.72	0				
0815	/	1			146.54			3.32		~ 118		
0816	/	2			146.81			3.59				
0817	/	3			147.05			3.83				
0818	/	4			147.24			4.02				
0820	/	6			147.72			4.50				
0822	/	8			148.15			4.93				
0824	/	10			148.53			5.31		118		
0829	/	15			149.41			6.19				
0834	/	20			150.13			6.71			clean discharge	
0839	/	25			150.84			7.62			minor sand	
0844	/	30			151.49			8.27		118		
0849	/	35			152.08			8.86				
0859	/	45			153.45			10.23				
0909	/	55			154.54			11.32				
0919	/	65			155.34			12.12		115		
0929	/	75			156.17			12.75		115		
0934	/	80			156.66			13.44		115	raised R.P.M.	
0944	/	90			157.78			14.56		118		
0954	/	100	0		158.49		154.99	15.27		118		
0955	/	101	1		160.42			17.20		~ 162	increase Q	
0956	/	102	2		160.62			17.40				
0958	/	104	4		160.95			17.73				
1000	/	106	6		161.23			18.01		162		
1002	/	108	8		161.66			18.44				
1004	/	110	10		161.95			18.73		162		
1009	/	115	15		162.61			19.39		162		
1014	/	120	20		163.25			20.02				
1020	/	125.5	25.5		164.00			20.77				
1024	/	130	30		164.45			21.23		162	cond=500 μ mhos @ 58°F	
1034	/	140	40		165.73			22.51				
1044	/	150	50		166.73			23.51		162		
1054	/	160	60		167.69			24.47				
1104	/	170	70		168.71			25.49				
1114	/	180	80		169.48			26.26		162		
1124	/	190	90		170.55			27.30				
1134	/	200	100/0		171.52		168.02	28.30				
1135	/	201	0							~ 209	increased Q	
1136	/	202	1		173.90			30.68				
1137	/	203	2		174.16			30.94		209		

PAGE 2 OF 2

PUMP OFF: date 12/16/85 time 1455





WASHOE COUNTY

DEPARTMENT OF PUBLIC WORKS
UTILITY DIVISION

PUMPING TEST DATA

WELL Horizon Hills

~~PUMPING~~ OBSERVATION WELL
PUMPING/RECOVERY DATA

PAGE 1 OF 2

TYPE of PUMPING TEST Constant Discharge

HOW Q MEASURED orifice plate / manometer

HOW WL's MEASURED electric sounder

M.P. for WL's _____ elev. _____

DEPTH of PUMP/AIRLINE 350 ft wrl _____

PUMPED WELL NO. _____

% SUBMERGENCE: initial _____; pumping _____

RADIUS of PUMPED WELL _____

PUMP ON: date 12/17/85 time 0820

DISTANCE from PUMPED WELL _____

PUMP OFF: date 12/20/85 time 0800

TIME					WATER LEVEL DATA					WATER PRODUCT.		COMMENTS
t = of t = 0					STATIC WATER LEVEL 151.57							
CLOCK TIME	ELAPSED TIME			t/t'	READING	CONVERSIONS or CORRECTIONS	WATER LEVEL	S or S'		Q	(NOTE ANY CHANGES BY OBSERVERS)	
	mins	hrs	sec									
0820	/	1			156.62			5.05				
0822	/	2			159.65			8.08				
0823	/	3			160.25			8.68				
0824	/	4			160.63			9.06				
0825	/	5			161.09			9.52				
0826	/	6			161.52			9.95				
0827	/	7			161.91			10.34				
0828	/	8			162.26			10.69		209		
0829	/	9			162.61			11.04				
0830	/	10			162.94			11.37				
0832	/	12			163.60			12.03				
0834	/	14			164.21			12.64				
0836	/	16			164.81			13.24		209		
0838	/	18			165.39			13.82			Sandy, cloudy discharge	
0840	/	20			165.92			14.35				
0845	/	25			167.16			15.59			Sample taken	
0850	/	30			168.37			16.80				
0855	/	35			169.50			17.93				
0900	/	40			170.55			18.98			Cond = 480 μ mhos @ 58°F	
0905	/	45			171.53			19.96				
0910	/	50			172.49			20.92				
0920	/	60			174.30			22.73		207		
0930	/	70			175.96			24.39		209		
0940	/	80			177.92			26.35		209		
0950	/	90			179.46			27.89				
1000	/	100			180.81			29.24				
1020	/	120			183.35			31.78		209		
1040	/	140			185.47			33.90				
1100	/	160			187.42			35.85				
1130	/	190			188.65			37.08		207		
1200	/	220			190.95			39.38		186	lowered Q	
1210	/	230			191.14			39.57		186		
1230	/	250			191.73			40.16		184		
1300	/	280			192.76			41.22		184		
1330	/	310			193.82			42.25				
1400	/	340			194.46			42.86				
1430	/	370			195.36			43.79				
1500	/	400			196.20			44.63		184		
1530	/	430			196.93			45.36				
1600	/	460			197.76			46.19		184		

WASHOE COUNTY

**DEPARTMENT OF PUBLIC WORKS
UTILITY DIVISION**

PUMPING TEST DATA

WELL Horizon Hills

**PUMPING/OBSERVATION WELL
PUMPING/RECOVERY DATA**

PAGE 2 OF 2

TYPE OF PUMPING TEST Constant Discharge

HOW Q MEASURED

HOW WL'S MEASURED _____

PUMPED WELL NO. _____

RADIUS of PUMPED WELL _____

DISTANCE from PUMPED WELL _____

M.P. for WL's _____ day _____

DEPTH of PUMP/AIRLINE _____ WT! _____

% SUBMERGENCE: initial _____; pumping _____

PUMP ON: date 12/17/85 time 0820

PUMP OFF: date 12/20/85 time 0800

TIME					WATER LEVEL DATA				WATER PRODUCT.		COMMENTS
at t'=0					STATIC WATER LEVEL 151.57						
CLOCK TIME	ELAPSED TIME		t/t'		READING	CONVERSIONS or CORRECTIONS	WATER LEVEL	S or S'		Q	
1630	hrs	mins	t	t'	198.27			46.70		184	
1700					199.95			48.38		184	cond = 460 μ mhos @ 56°F
1740					199.45			47.88		182	adj Q
1820					200.97			49.40		184	
1900					201.48			49.91			
2000					202.46			50.89			sample #2
2100					203.30			51.73		184	
2200					204.00			52.43		182	adj Q
2330					204.90			53.33		184	
0100					205.55			53.98		184	
0230					206.26			54.69		184	
0400					206.95			55.38		"	
0530					207.64			56.07		184	
0700					208.15			56.58		178	adj. Q
0810										186	sample #3
0900					209.14			57.57		186	cond = 480 μ mhos @ 58°F
1100					210.21			58.64		"	
1300					211.93			60.36		"	
1530					211.74			60.17		"	
1930					212.85			61.28		186	adj. Q to
2100					213.37			61.80		186	sample #4, cond = 470 μ mhos @ 58°F
2300					213.90			62.33		"	
0100					214.34			62.77		186	
0315					215.00			63.43		186	
0520					215.58			64.01		184	adj Q
0745					216.11			64.54		"	sample #5
1045					217.05			65.48		186	
1245					217.50			65.93		"	
1445					217.92			66.35		184	Health sample
1900					219.04			67.47		"	
2130					219.80			68.23		184	cond = 470 @ 58°F; #6
2330					220.10			68.53		182	adj R.P.M.
0100					221.25			69.68		186	
0404					222.04			70.46		186	
0745					223.28			71.71		190	sample #7, cond = 470
0800											end test; Health sample; temp = 58°

WASHINGTON COUNTY

**DEPARTMENT OF PUBLIC WORKS
UTILITY DIVISION**

PUMPING TEST DATA

WELL Horizon Hills

PUMPING/OBSERVATION WELL
PUMPING/RECOVERY DATA

PAGE 1 OF 1

TYPE OF PUMPING TEST Constant Discharge Recovery

HOW Q MEASURED _____

HOW WL'S MEASURED electric sounder

PUMPED WELL NO. _____

RADIUS of PUMPED WELL _____

DISTANCE from PUMPED WELL _____

M.P. for WL's _____ day _____

DEPTH of PUMP/AIRLINE _____ WTI _____

% SUBMERGENCE: initial _____; pumping _____

PUMP ON: date 12/20/85 time 0800

PUMP OFF: date _____ time _____

[illegible]

750

NEVADA DIVISION OF HEALTH
1660 N. Virginia Street
Reno, Nevada 89503
(702) 789-0335

70250

WATER CHEMISTRY ANALYSIS:

Attention: Fees may apply to some types of samples.

TYPE OF ANALYSIS:

- ☒ Check here for ROUTINE DOMESTIC ANALYSIS.
☐ Circle the constituents needed for PARTIAL ANALYSIS.

SAMPLING INSTRUCTIONS:

The sample submitted must be representative of the source. Spring and surface water samples should be as free of dirt and debris as possible. Wells should be pumped thoroughly before sampling, changing the water in the casing at least three times. Product water from filters should be sampled after running for about ten (10) minutes.

Sampled by Clayton Date 12/19/85
Owner Horizon Hills G.D. Phone _____
Address N. Virginia
City Reno State NV

REPORT TO:

Name WCDAD
Address 9th & Wells
City Reno
State NV Zip _____

All of the information below must be filled in or the analysis will not be performed.

State NV County Washoe
Township 20 Range 17 Section 17
General Location _____
Source Address Horizon Hills

REASON FOR ANALYSIS:

- ☐ Loan
☐ Personal health reasons
☐ Purchase of the property
☐ Rental or sale of property
☐ Subdivision approval
☒ Other SDWA

USE OF WATER:

- ☒ Domestic drinking water
☐ Geothermal
☐ Industrial or mining
☐ Irrigation
☐ Other _____
Initials _____

SOURCE OF WATER:

Filter ☐ Yes ☐ No
Public ☒ Yes ☐ No
Spring _____
Well X Depth _____ ft.
Hot _____ Cold X
IN USE ☐ Yes ☒ No
Type _____
Name _____
Surface _____
Casing diameter 12 in.
Casing depth _____ ft.

EPM: 0.0240

The results below are representative only of the sample submitted to this laboratory.

250

S: 895 R: 0.7032				FOR LABORATORY USE ONLY		EC: 529		PRINT OTHER DESIRED CONSTITUENTS BELOW	
Constituent	ppm	Constituent	ppm	Constituent	ppm	Constituent	S.U.	Constituent	ppm
T.D.S. @ 103° C.	372	Chloride	2	Iron	0.73	Color	7	Cl	
Hardness	228	Nitrate	0.4	Manganese	0.23	Turbidity	1.5	CH	
Calcium	50	Alkalinity	92	Copper	0.01	pH	7.54	PL	
Magnesium	25	Bicarbonate	112	Zinc	0.02			Hg	
Sodium	25	Carbonate	0	Barium	0.02			Se	
Potassium	1	Fluoride	0.27	Boron	0.0			Ag	
Sulfate	179	Arsenic	< 0.003	Silica	30				
MBAS	< 0.1	NO3	14th	0.09	BA	from			

pen
D. Coulter
2/19/85

Fee _____
Collected by _____
PWS ID _____
SD _____
1st _____ 2nd _____ 3rd _____
Date Rec'd _____ Init _____
ppm = parts per million, milligrams per liter
S.U. = Standard Units

Remarks 56 hr pump test 1400
1/3/85
Circled items exceeds the U.S.P.H.S. Drinking Water Stds. The limits are:
9/11/85



TERRA ENVIRONMENTAL MONITORING

GENERAL MINERAL ANALYSIS REPORT

CLIENT: Washoe County - Dept of Public Works Utilities Division P. O. Box 11130 Reno, NV 89520	DATE OF REPORT			JOB NUMBER WAS - 314
	MON	DAY	YR.	
	11 - 13 - 85			

SAMPLE IDENTIFICATION DATA

IDENTIFICATION NUMBER: Horizon Hills - New Well
SAMPLE COLLECTION DATE: 11 - 4 - 85
SAMPLE COLLECTION TIME: 09:45 HRS
SAMPLE COLLECTED BY: Mike Widmer
SAMPLING LOCATION: Horizon Hills
Washoe County, NV

Preliminary Results

P. O. # 70614

PARAMETER	VALUE	CRITERIA	PARAMETER	VALUE	CRITERIA
BICARBONATE mg/l	90	—	MANGANESE mg/l	0.16	0.10 ₂
ALKALINITY mg/l	0	—	NITRATE NO_3 mg/l	1.7	45 ₁
CARBONATE mg/l	0	—	PH	7.7	6.5 - 8.5 ₂
CALCIUM mg/l	35	—	SODIUM mg/l	36	—
CHLORIDE mg/l	4	400 ₂	SULFATE mg/l	130	500 ₂
ARSENIC mg/l	0.003	0.05	TOTAL DISSOLVED SOLIDS mg/l	322	1000 ₂
FLUORIDE mg/l	0.2	1 ₁	TOTAL HARDNESS mg/l	154	—
IRON mg/l	0.55	0.6 ₂			
MAGNESIUM mg/l	16	150 ₂			

OTHER WATER QUALITY ANALYSIS REQUESTED

Copper mg/l	<0.02				

- * CRITERIA DEPENDENT ON LOCAL ANNUAL AVERAGE MAXIMUM DAILY TEMPERATURES
1. U.S. ENVIRONMENTAL PROTECTION AGENCY PRIMARY DRINKING WATER STANDARDS.
 2. STATE OF NEVADA DIVISION OF HEALTH SECONDARY DRINKING WATER STANDARDS.